

GENERAL NOTES:

GENERAL

- 1. THESE GENERAL NOTES PRESENT AND/OR SUMMARIZE KEY PROJECT INFORMATION FOR THE PLAN READER'S CONVENIENCE. SEE PLANS AND SPECIFICATIONS FOR FURTHER REQUIREMENTS.
2. ALL REFERENCES TO STANDARDS HEREIN ARE TO MOST RECENT ISSUE IN EFFECT AS OF THE DATE OF THESE DOCUMENTS, UNLESS NOTED OTHERWISE IN PROJECT SPECIFICATIONS.
3. DESIGN BASIS: 2018 INTERNATIONAL BUILDING CODE (IBC)
A. GENERAL RISK CATEGORY = III
B. WIND: ULTIMATE DESIGN WIND SPEED = 127 MPH
WIND EXPOSURE CATEGORY = B
INTERNAL PRESSURE COEFFICIENT = 0.18 +/- (ENCLOSED BUILDING)
COMPONENT & CLADDING DESIGN PRESSURE - SEE DIAGRAMS BELOW.

COMPONENTS & CLADDING
INTERNAL PRESSURE LOADS (psf)
EFFECTIVE WIND AREA (SQ FT) ASCE 7-16, CHAPTER 30
ROOF WALLS
Table with wind speed, area, and pressure values.
GABLE ROOF PLAN
WALL ELEVATION
NOTES:
1. a = 6'-0"
2. POSITIVE PRESSURE VALUES REFER TO FORCES ACTING TOWARDS BUILDING...
3. EACH COMPONENT MUST BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE FORCES.
4. FOR COMPONENTS HAVING EFFECTIVE AREAS IN BETWEEN TABULATED VALUES...
5. THE FINAL NET DESIGN WIND PRESSURE, INCLUDING ALL PERMITTED REDUCTIONS, SHALL NOT BE LESS THAN 16 psf ACTING IN EITHER DIRECTION.

- C. SEISMIC: SEISMIC IMPORTANCE FACTOR Ie = 1.5
MAPPED SPECTRAL RESPONSE ACCEL (SHORT PERIODS) Ss = 0.35
MAPPED SPECTRAL RESPONSE ACCEL (1 SECOND PERIOD) S1 = 0.24
SPECTRAL RESPONSE COEFFICIENT (SHORT PERIODS) SDS = 0.284
SPECTRAL RESPONSE COEFFICIENT (1 SECOND PERIOD) SD1 = 0.156
SEISMIC DESIGN CATEGORY = D
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
DESIGN BASE SHEAR = PER PEMB MANUFACTURER
D. LIVE LOADS: ROOF: 20 psf
MEZZANINE: 125 psf
COLLATERAL LOAD: 10psf
E. SNOW LOAD: GROUND: 5 psf

ABBREVIATIONS table with columns for symbol, description, and material type. Includes symbols for TOP (BAR), BOTTOM (BAR), INTERIOR, EXTERIOR, ELEVATION, ON CENTER, EACH WAY, NEAR SIDE, FAR SIDE, WORK POINT, FINISH, FLOOR, CLEAR, TOP OF, BOTTOM OF, WITH, GAGE/GAUGE, EQUAL, FOOTING, TYPICAL, JOIST, REINFORCING, TRUSS, STEEL, WOOD, CONCRETE, MASONRY, LIGHT GAGE, APPROXIMATE, SPACES/SPECS, UNLESS NOTED OTHERWISE, PLCS.

- 5. UNLESS OTHERWISE NOTED, REQUIREMENTS GIVEN FOR ONE LOCATION ALSO APPLY AT OTHER LOCATIONS AT WHICH CONDITIONS ARE SIMILAR. THE REQUIREMENTS GIVEN SHALL BE ADAPTED TO SIMILAR CONDITIONS AT SIMILAR LOCATIONS.
6. COORDINATE WORK OF OTHER TRADES SHOWN ON DRAWINGS OR INDICATED IN SPECIFICATIONS WITH STRUCTURAL WORK.
7. SHOP DRAWINGS FOR ANY PART OF THE STRUCTURAL WORK SHALL SHOW THE INTERFERENCE WITH OTHER RELATED TRADES. THE CONTRACTOR SHALL VERIFY DIMENSIONS, LOCATIONS, MATERIALS, ETC. OF RELATED TRADES BY CERTIFIED MANUFACTURERS' DRAWINGS AND SHALL INDICATE THEREON SUBMITTING SHOP DRAWINGS FOR ARCHITECT/ENGINEER'S APPROVAL.
8. THE DESIGN OF THE STRUCTURE SHOWN IS BASED ON INTERACTION OF MEMBERS CONNECTED PARTS AND THE DESIGN LOADS NOTED ABOVE. THE STRENGTH AND STABILITY OF CONSTRUCTION UNDERWAY MAY REQUIRE SUPPLEMENTAL TEMPORARY SUPPORT BRACING OR OTHER MEASURES. THE CONTRACTOR SHALL DETERMINE THE NEED OF SUCH TEMPORARY SUPPORT DURING CONSTRUCTION AND PROVIDE ALL SUCH MEASURES.

EARTHWORK/FOUNDATION

- 1. FOUNDATION DESIGN BASIS: CSRA TESTING & ENGINEERING, REPORT #B-008.20. ALLOWABLE BEARING CAPACITY IS 2,000 PSF, MAXIMUM.
2. NO BLASTING WILL BE ALLOWED.
3. CONTROL OF GROUND WATER, IF REQUIRED, SHALL BE ACCOMPLISHED IN A MANNER THAT WILL PRESERVE THE STRENGTH OF THE FOUNDATION SOILS, WILL NOT CAUSE INSTABILITY OF THE EXCAVATION SLOPES, AND WILL NOT RESULT IN DAMAGE TO EXISTING STRUCTURES.
4. COORDINATE FOUNDATION WORK WITH ALL OTHER TRADES.
5. PIPES AND OTHER WORK WHICH REQUIRE EXCAVATING OR TRENCHING ADJACENT TO COLUMN FOOTINGS OR PARALLEL TO WALL FOOTINGS, SHALL NOT BE LOCATED BELOW LINES EXTENDING DOWNWARD FROM THE BOTTOM EDGE OF THE FOOTING AT A 45 DEGREE ANGLE FROM HORIZONTAL.
6. EXCAVATIONS FOR FOOTINGS, GRADE BEAMS, MATS AND OTHER FOUNDATIONS BUILT NEXT TO OR AROUND EXISTING FOUNDATIONS, SHALL NOT EXTEND BELOW THE BOTTOM SURFACE OF THE EXISTING FOOTING UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DESIGN DRAWINGS. HOLES ADJACENT TO EXISTING FOOTINGS (CLOSER TO THE FOOTING EDGE THAN THE HOLE DEPTH) CAN NOT BE OVER-EXCAVATED AND FILLED TO ACCOUNT FOR BAD SOIL UNLESS SPECIFICALLY APPROVED BY THE ENGINEER OF RECORD.
7. ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS INCLUDING ELEVATION, SIZE AND THICKNESS OF FOUNDATIONS SHALL BE INDICATED BY THE GENERAL CONTRACTOR ON THE REINFORCING SHOP DRAWINGS. SUCH PROPOSED DEVIATIONS SHALL BE CIRCLED AND NOTED "ENGINEER VERIFY".
8. STRUCTURAL FILL SHALL BE PLACED IN LIFTS NO MORE THAN 8" THICK WITH A COMPACTION OF 95% STANDARD PROCTOR (PER ASTM D-698) MAXIMUM DRY DENSITY.

CONCRETE

Table with columns: STRENGTH (PSI), AIR (%), CEMENT (# MIN), W/C RATIO, SLUMP, AGGREGATE (MAX.), LOCATION. Includes rows for 2,000, 3,000, 4,000, and 2,500 PSI concrete with details on mix design and aggregate.

- 1. THE QUANTITY OF CEMENT REPLACED SHALL BE NO MORE THAN 20%.
2. CEMENT SHALL BE REPLACED BY FLY ASH AT THE RATE OF 1.25 LBS. OF FLY ASH TO 1.0 LBS OF CEMENT.
C. ALL CONCRETE DELIVERED TO THE SITE SHALL HAVE A COMPUTER BATCH WEIGHT TICKET. THE TICKET SHALL SHOW WEIGHTS OF ALL MATERIALS, VOLUME OF CONCRETE AND TIME OF BATCH. THE BATCH WEIGHT TICKET SHALL BE GIVEN TO A DESIGNATED OWNER'S REPRESENTATIVE AT THE TIME OF DELIVERY FOR VERIFICATION OF MIX PROPORTIONS.
D. CONSOLIDATE ALL CONCRETE IN FORMS AND TRENCHES WITH VIBRATION. POORLY CONSOLIDATED CONCRETE WILL BE REJECTED AND REPLACED AT CONTRACTOR'S EXPENSE.

- 2. CONCRETE REINFORCING
A. ALL REINFORCING SHALL BE PER ASTM A-615, GRADE 60.
B. WELDING OF REINFORCING STEEL IS NOT PERMITTED.
C. REINFORCING SHALL NOT BE HEATED TO BEND.
D. WELDED WIRE FABRIC SHALL BE PER ASTM A-185.

- 3. SUBMITTALS
A. CONCRETE MIX DESIGNS, SHOP DRAWINGS FOR CONCRETE REINFORCING, EMBEDDED ITEMS, ACCESSORIES, AND PRODUCT DATA, ETC. AS OUTLINED IN THE SPECIFICATIONS SHALL BE PROVIDED TO THE OWNER'S REPRESENTATIVE AT LEAST 15 DAYS PRIOR TO THE START OF WORK FOR APPROVAL.
B. ALL DATA SHALL BE SUBMITTED "CONTRACTOR APPROVED".
NOTIFICATIONS: THE CONTRACTOR SHALL NOTIFY THE OWNER
A. WHEN EXCAVATION TO REQUIRED SUBGRADE ELEVATIONS IS REACHED.
B. 4 HOURS PRIOR TO ANY SCHEDULED CONCRETE PLACEMENT FOR INSPECTION OF FORMWORK, REINFORCING AND EMBEDDED ITEMS.

MASONRY WALL REINFORCING/JOINTS

- 1. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH TMS 402/602-16 AND THE FOLLOWING:
2. THE REINFORCING, JOINTS AND CRITERIA DESCRIBED IN THE FOLLOWING GENERAL NOTES ARE REQUIRED AS A MINIMUM FOR ALL RUNNING BOND MASONRY WALLS. SEE SPECIFIC CRITERIA ON DESIGN DRAWINGS FOR ANY ADDITIONAL REQUIREMENTS AND/OR STACK BOND CRITERIA.
3. VERTICAL REINFORCING (RUNNING BOND):
A. PROVIDE REINFORCING STEEL IN A CONCRETE FILLED CELL CONTINUOUS FROM FOOTING INTO BOND BEAM AT TOP OF WALL. LAP BARS WITH FOOTING DOWEL AND EXTEND 4" MINIMUM INTO BOND BEAM BREAK-OUT BOTTOM OF BOND BEAM AT FILLED CELL LOCATIONS AND FILL TOP FOUR COURSES OF WALL WHEN BOND BEAM IS FILLED. PROVIDE AT THE FOLLOWING LOCATIONS:
- AT ALL WALL CORNERS
- WITHIN 8" OF ENDS OF ALL WALLS AND AT EACH SIDE OF EXPANSION AND CONTROL JOINTS.
- AT ALL DOOR AND WINDOW JAMBS AND AT ALL OPENINGS GREATER THAN 16" IN WIDTH ALONG ENTIRE LENGTH OF ALL WALLS AS NOTED IN THE TABLE BELOW.

Table for VERTICAL FOR SEISMIC DESIGN CATEGORY D and MASONRY REINFORCING LAP LENGTHS. Includes columns for WALL WIDTH and reinforcement specifications for non-load bearing and load bearing walls.

- 4. HORIZONTAL REINFORCING (RUNNING BOND):
A. PROVIDE HORIZONTAL JOINT REINFORCING AS NOTED IN THE TABLE BELOW:
Table for HORIZONTAL FOR SEISMIC DESIGN CATEGORY D.
B. PROVIDE CONCRETE FILLED BOND BEAM WITH 2 #5 REBARS CONTINUOUS WHERE WALLS ARE STRUCTURALLY CONNECTED TO ROOF AND FLOOR LEVELS AND AT THE TOPS OF ALL WALLS.
C. PROVIDE CONCRETE FILLED COURSE WITH 1 #4 REBAR AT DOOR AND WINDOW HEADS, AND AT ALL WINDOW SILL HEADS. EXTEND THE GREATER OF 2'-0" OR 40 BAR DIAMETERS BEYOND OPENING.

- 5. CONTROL JOINTS:
A. CONTROL JOINTS SHALL BE LOCATED IN ALL WALLS AT THE FOLLOWING LOCATIONS:
- AT A MAXIMUM SPACING OF 3 TIMES THE WALL HEIGHT, BUT NOT GREATER THAN 40'-0" ON CENTER.
- AT A DISTANCE OF NOT OVER ONE TIME THE WALL HEIGHT FROM BUILDING CORNERS.
- AT ALL CHANGES IN WALL HEIGHT.
- AT ALL CHANGES IN WALL THICKNESS, SUCH AS AT PIPE OR DUCT CHASES, AND ADJACENT TO STEEL COLUMNS EMBEDDED IN WALLS AND PILASTERS.
- ABOVE JOINTS IN FOUNDATIONS AND IN FLOORS.
- BELOW JOINTS IN FLOORS OR ROOFS THAT BEAR ON THE WALL.

- 6. ISOLATION JOINTS SHALL BE LOCATED WHERE NON-LOAD BEARING WALLS ABUT LOAD BEARING WALLS OR SHEAR WALLS.
7. WALL BRACING:
A. ALL NON-LOAD BEARING MASONRY WALLS, (FULL HEIGHT AND PARTIAL HEIGHT) SHALL BE BRACED ALONG ENTIRE LENGTH. BRACE POINTS SHALL OCCUR AT CONCRETE FILLED VERTICAL CELLS AS DEFINED IN NOTE 2.A ABOVE.

- LOAD-BEARING METAL STUD NOTES:
1. UNLESS NOTED OTHERWISE, ALL STUDS SHALL BE EQUAL TO A MINIMUM OF 18 GA. SPACED AT 16" CENTERS WITH 18 GA. TRACK, TOP AND BOTTOM.
2. MINIMUM YIELD STRENGTH (Fy) FOR STUDS IS 33,000 p.s.i. FOR 18 GA. AND 20 GA. MATERIALS, AND 50,000 p.s.i. FOR 16 GA., 14 GA., AND 12 GA. MATERIALS.
3. ALL STUDS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A G-60 GALVANIZED COATING MEETING THE REQUIREMENTS OF A.S.T.M. A653.
4. STUDS SHALL HAVE FULL BEARING AGAINST THE INSIDE TRACK WEB TOP AND BOTTOM. STUDS MUST BE CUT SQUARE.
5. BRIDGING IS TO BE SPACED AT NO MORE THAN 4'-0" O.C. VERTICALLY.
6. MINIMUM TRACK FASTENING SHALL BE 0.177" DIAMETER POWDER ACTUATED FASTENERS SPACED ON 12" CENTERS FOR BEARING WALLS AND AT 16" O.C. FOR NON-LOAD BEARING WALLS (U.N.O.), WITH 1 1/2" MINIMUM PENETRATION INTO CONCRETE.
7. VOIDS BENEATH TRACK SHALL NOT BE PERMITTED. CONTRACTOR SHALL PROVIDE A REASONABLY LEVEL SLAB WITH A TOLERANCE OF 1/8" IN 10 FEET. WHERE UNEVENNESS OF SUPPORTING FLOOR PREVENTS CONTINUOUS SOLID BEARING, PANEL OR TRACK SHALL BE LEVELLED BY PLACING MORTAR OR GROUT BENEATH TRACK.
8. CONTINUOUS STUDS EACH SIDE OF HEADERS SHALL BE EQUAL TO 1/2 OF THE INTERRUPTED STUDS PLUS ONE STUD AT EACH SIDE. USE MINIMUM OF TWO (2) STUDS EACH SIDE. HEADERS SHALL BE DESIGNED TO TRANSFER ALL UNIFORM AND/OR CONCENTRATED LOADS. SHEAR SHALL BE TRANSFERRED BY FULL BEARING ON JACK STUDS OR BY SHEAR PLATES. SHEAR PLATES SHALL BE 16 GA. MINIMUM.
9. CUTTING OF LOAD-BEARING METAL STUDS IS NOT PERMITTED WITHOUT SPECIFIC APPROVAL FROM THE ENGINEER OF RECORD.

STRUCTURAL STEEL

- 1. INSTALLATION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH DIVISION 5 OF THE SPECIFICATIONS AND THE FOLLOWING:
2. STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS", 15TH EDITION, 2016.
3. STEEL FABRICATOR SHALL PARTICIPATE IN THE AISC QUALITY CERTIFICATION PROGRAM AND BE DESIGNATED AS AISC-CERTIFIED PLANT, CATEGORY STD.
4. UNLESS NOTED OTHERWISE STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:
A. STRUCTURAL (W, S, T, I OR H) BEAMS AND COLUMNS - ASTM A-572 GRADE 50 OR ASTM A992
B. STRUCTURAL (C OR MC) CHANNELS AND ANGLES - ASTM A-36
C. MISCELLANEOUS PLATES, BARS AND ANGLES - ASTM A-36
D. ANCHOR BOLTS AND RODS - ASTM A-36 OR ASTM F1554, GRADE 36
E. COLD-FORMED HOLLOW STRUCTURAL SECTIONS (HSS) - ASTM A500, GRADE B, STRUCTURAL TUBING
F. STRUCTURAL PIPE - ASTM A53, TYPE E OR S, GRADE B, STANDARD WEIGHT, UNLESS NOTED OTHERWISE ON DRAWINGS.
5. UNLESS NOTED OTHERWISE BOLTED CONNECTIONS SHALL CONFORM TO THE FOLLOWING:
A. HIGH STRENGTH BOLTS - 3/4" DIAMETER ASTM F3125 TYPE 3, 325 TYPE 1 HEAVY-HEX
B. NUTS - HEAVY-HEX ASTM A563, GRADE C
C. WASHERS - ASTM F436 TYPE 1, HARDENED (RCSC SPEC TABLE 10.2 PART 14 FOR ANCHOR RODS)
D. BOLT, NUT AND WASHER FINISH SHALL BE THE FINISH OF THE STEEL IT CONNECTS.
6. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL BE SHOP PRIMED.
7. MINIMUM SIZE WELD SHALL BE FULL PENETRATION GROOVE WELDS. ALL WELDS SHALL BE PERFORMED BY CERTIFIED WELDERS AND CONFORM TO REQUIREMENTS OF AWS D1.1.
8. MINIMUM MATERIAL THICKNESS SHALL NOT BE LESS THAN 3/8" FOR MISCELLANEOUS PLATES.
9. INSTALL CORNERS PLATE BENEATH STEEL WEDGES AT EDGES OF BASE PLATE TO PROVIDE FIRM BEARING. GROUT FOR SETTING PLATES SHALL BE NON-SHRINK, NON-METALLIC. WHEN GROUT HAS GAINED SUFFICIENT STRENGTH TO SUPPORT LOAD, ALL WEDGES AND SHIMS SHALL BE REMOVED AND RESULTING VOIDS FILLED WITH GROUT.
10. ALIGN AND ADJUST VARIOUS MEMBERS THAT FORM PART OF A STEEL STRUCTURE BEFORE PERMANENTLY FASTENING. MAINTAIN ERECTION TOLERANCES OF STRUCTURAL STEEL WITHIN AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
DO NOT USE THERMAL CUTTING DURING ERECTION OR ENLARGE HOLES BY BURNING.
12. CLEAN AND REPAIR FINISHES DAMAGED DURING ERECTION.
13. SUBMITTALS
A. SHOP DRAWINGS AND MATERIAL SUBMITTALS SHALL BE REQUIRED FOR STRUCTURAL AND MISCELLANEOUS STEEL, ACCESSORIES, AND PRODUCT DATA, ETC., AS OUTLINED IN THE SPECIFICATIONS.
B. ALL DATA SHALL BE SUBMITTED "CONTRACTOR APPROVED".

ELEVATED METAL DECK

- 1. UNLESS NOTED OTHERWISE, THESE METAL DECK NOTES APPLY TO NON-COMPOSITE AND COMPOSITE METAL FLOOR DECKING TOPPED WITH CONCRETE, AND UN-TOPPED METAL ROOF DECKING.
2. INSTALLATION OF ELEVATED METAL DECK SHALL BE IN ACCORDANCE WITH DIVISION 5 OF THE SPECIFICATIONS, SDINC-2017 AND THE FOLLOWING:
3. STEEL DECK SHALL BE MANUFACTURED AND ERECTED IN ACCORDANCE WITH THE STEEL DECK INSTITUTE. ALL DECKING SHALL BE GALVANIZED, UNLESS NOTED OTHERWISE ON THE PLANS.
4. CONTRACTOR SHALL PROVIDE ACCESSORIES REQUIRED TO COMPLETE THE METAL DECK. INSTALLATION AND THE CONCRETE PLACEMENT INCLUDING (BUT NOT LIMITED TO) CELL AND COLUMN CLOSURES, POUR STOPS AND BEAM FILLERS.
5. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS, THE CONTRACTOR SHALL INSTALL AN EDGE ANGLE OR BENT PLATE AROUND THE DECKING PERIMETER AND AT INTERIOR OPENINGS. THE ANGLE OR BENT PLATE SHALL BE A MINIMUM 1/4 INCH THICK WITH A HEIGHT THAT MATCHES THE SLAB THICKNESS FOR FLOOR DECKING OR 4 INCHES FOR ROOF DECKING.
6. METAL DECK ENDS WHICH ABUT A CONCRETE OR CMU WALL (AND ARE NOT SUPPORTED BY STEEL FRAMING WITHIN 6" OF THE DECK EDGE) SHALL BE SUPPORTED BY AN L4X3X1/4 LLV CONTINUOUS ANGLE ATTACHED TO THE WALL. METAL DECK SIDES ABUTTING A WALL DO NOT NEED ADDITIONAL SUPPORT UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS.
7. UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS, METAL ROOF DECKING SHALL BE ATTACHED TO THE SUPPORTING STRUCTURE WITH A 36/7 PATTERN WITHIN 12 FEET OF ANY BUILDING EDGE. FLOOR DECKING AND REMAINING ROOF DECKING SHALL BE ATTACHED WITH A 36/4 PATTERN. ATTACHMENT SHALL BE WITH #12 SELF-DRILLING SCREWS OR POWDER ACTUATED OR PNEUMATIC PINS.
8. METAL DECKING SHALL BE CONNECTED AT ITS SIDE LAPS WITH 2-#10 SCREWS MINIMUM. MAXIMUM SPACING OF SIDE LAP SCREWS SHALL BE 3'-0" UNLESS NOTED OTHERWISE IN THE DESIGN DRAWINGS.
9. EXACT LOCATION AND SIZES OF PENETRATIONS THROUGH FLOORS AND ROOFS SHALL BE COORDINATED WITH MECHANICAL AND ELECTRICAL DRAWINGS. FRAMING FOR MECHANICAL EQUIPMENT SHALL BE AS DETAILED ON THE DRAWINGS AND SHALL BE SUBMITTED FOR REVIEW. ALL OPENINGS GREATER THAN 12" IN DIAMETER OR 12" SQUARE SHALL BE REINFORCED BY ANGLE FRAMING.
10. ALL HVAC EQUIPMENT (FANS, ETC.) SHALL BE SUPPORTED BY STRUCTURAL STEEL FRAMING AND/OR ANGLE FRAMING. NO EQUIPMENT SHALL BE SUPPORTED DIRECTLY BY THE METAL DECK.
11. SUBMITTALS
A. SHOP DRAWINGS AND MATERIAL SUBMITTALS SHALL BE REQUIRED FOR ELEVATED STEEL DECK, ACCESSORIES, AND PRODUCT DATA, ETC., AS OUTLINED IN THE SPECIFICATIONS.
B. ALL DATA SHALL BE SUBMITTED "CONTRACTOR APPROVED".



CITY OF NORTH AUGUSTA, SC
100 GEORGIA AVENUE, NORTH AUGUSTA, SC 29841
PROJECT NAME: NORTH AUGUSTA FIRE STATION 1 RELOCATION
PROJECT LOCATION: 3111 W. MARTINTOWN ROAD, NORTH AUGUSTA, SC 29841

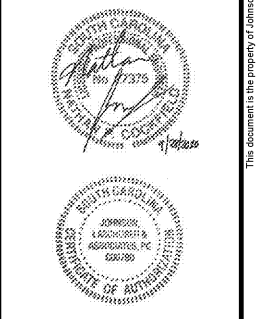


Table with columns: REV, DATE, BY, DESCRIPTION. Includes revision history for the drawing.

PROJECT NO: 3057.2003
DRAWN BY: BLB
CHECKED BY: NPC
DATE: 10/08/2020

SHEET TITLE: GENERAL NOTES
SCALE: AS NOTED
DRAWING NO: S-001
REV: 0

